

Exam CT4150 Plastic Analysis of Structures

Thursday 17 June 2009, 9:00 – 12:00 hours

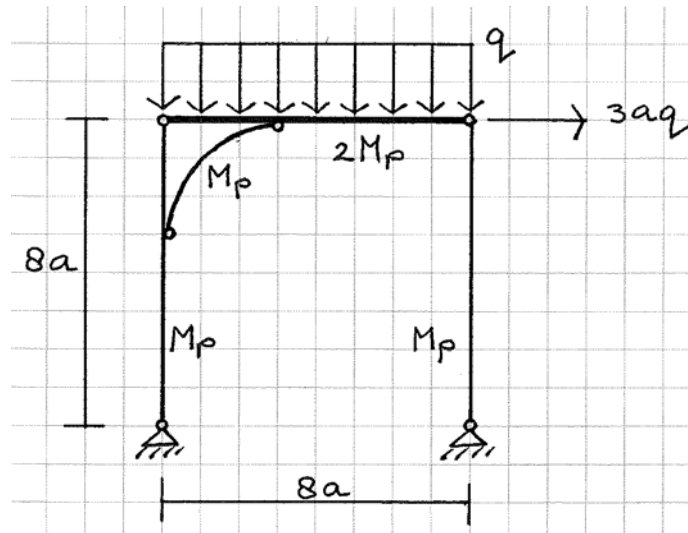


Figure 1. Frame

Problem 1

A frame consists of two columns, a beam and a curved bracket (Fig. 1). All joints between the members and with the foundation are pinned connections. The structure is loaded by a vertical evenly distributed load q and a horizontal load $3aq$. The following relation exists between the plastic moment M_p and the plastic normal force N_p (Fig. 2).

$$N_p = \beta \frac{M_p}{a}$$

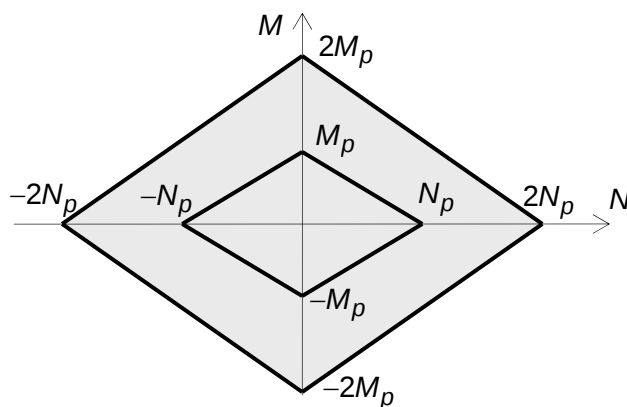


Figure 2. Yield contour

The influence of shear on the yield contour is neglected. Buckling and second order effects are not considered.

- a Assume $\beta \rightarrow \infty$. Determine the collapse load q for all possible mechanisms. Write the collapse loads as functions of M_p and a . What is the decisive collapse load? (1.5 point)
- b Assume $\beta \rightarrow \infty$. Draw the bending moment diagram and normal force diagram for the structure at the moment of collapse. (1.5 points)
- c Assume $\beta = 90$. Choose one of the following problems (You need not do both).
 – Determine the largest lower-bound for q .
 – Determine the smallest upper-bound for q .
 You only need to write down the equations and not solve the equations (1.5 points).

Problem 2

A reinforced concrete plate has fixed and simply supported edges (Fig. 3). It carries an evenly distributed load q . The plate is homogeneous and orthotropic.

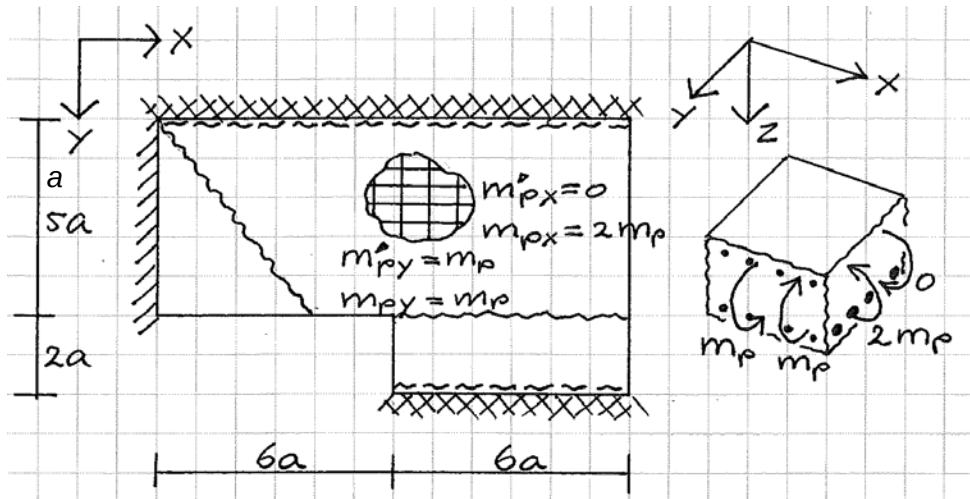


Figure 3. Plate dimensions, reinforcement and yield line pattern

- a Consider the yield line patterns of Figure 4. Which of these patterns give kinematically possible mechanisms. (1 point)
- b Consider the yield line pattern of Figure 3. Determine an upper bound for q expressed in m_p and a (1.5 point).
- c Determine the largest lower-bound for q using torsion free beams ($m_{xy} = 0$) (1.5 point).

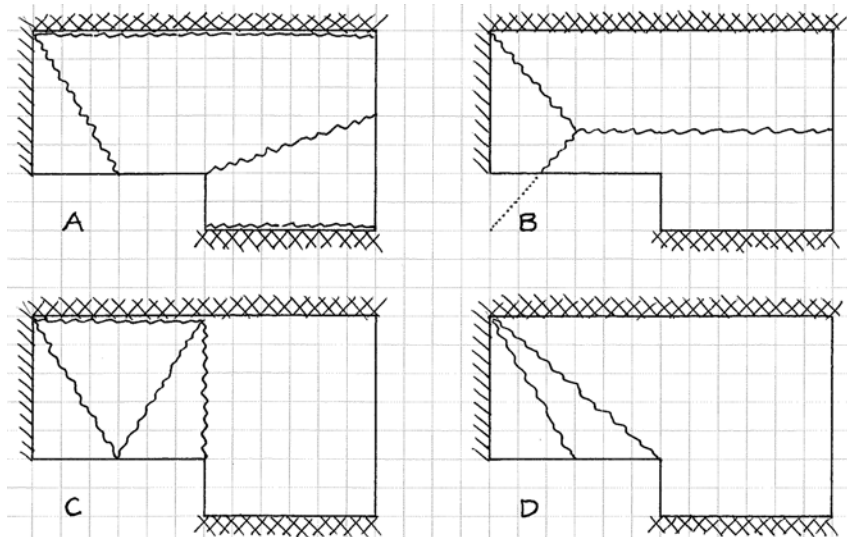


Figure 4. Yield line patterns of problem 2a

Problem 3

- a** Eurocode 2 uses strut-and-tie models for designing reinforced concrete structures. How are strut-and-tie models related to plastic analysis? Choose A, B, C, or D (0.5 point).

- A Strut-and-tie models are applications of the lower bound theorem.
- B Both the strut-and-tie method and plastic analysis require sufficient ductility of the material.
- C Nodes in strut-and-tie models are similar to plastic hinges in frames and yield lines in plates.
- D The strut directions need to be in the direction of the elastic stress trajectories. Therefore, there is no relation.

- b** Consider an elementary plate part with reinforcing moments

$$m_{px} = 17, m_{py} = 0, m'_{px} = 0, m'_{py} = 10 \text{ kNm/m}.$$

The plate part is loaded by the section moments

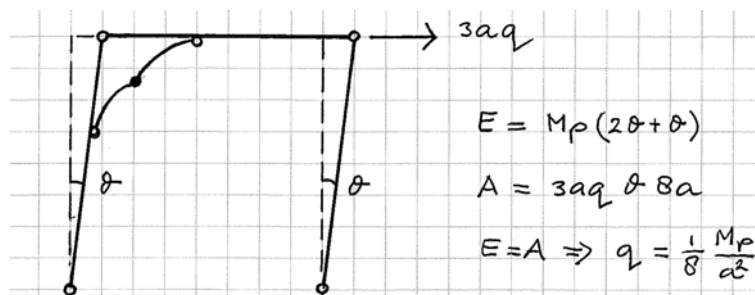
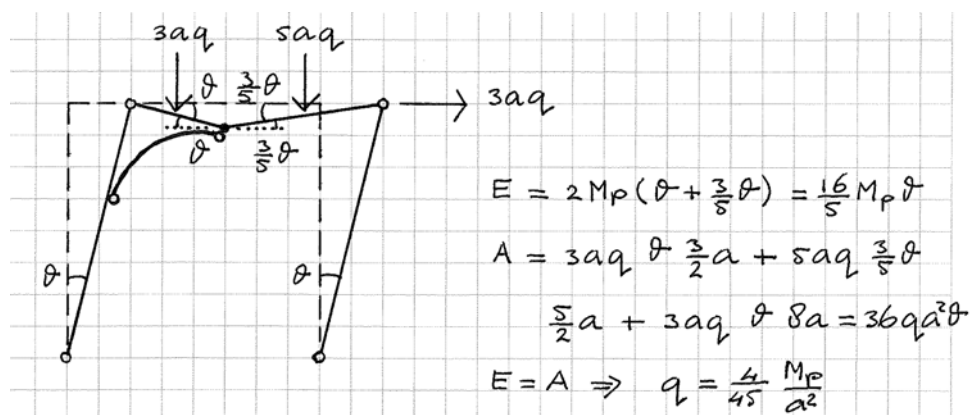
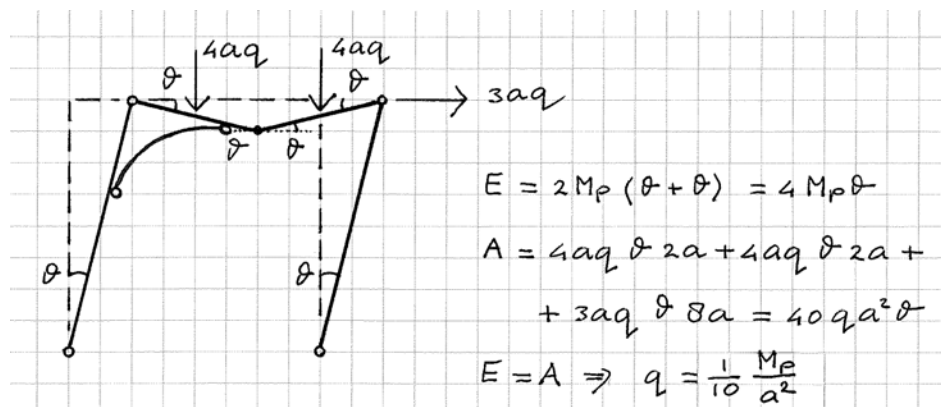
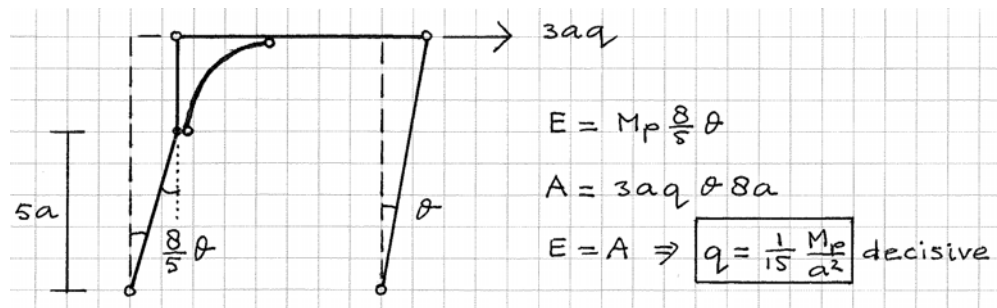
$$m_{xx} = 13, m_{yy} = -8 \text{ and } m_{xy} = 5 \text{ kNm/m}.$$

Will this plate part yield? Explain your answer.

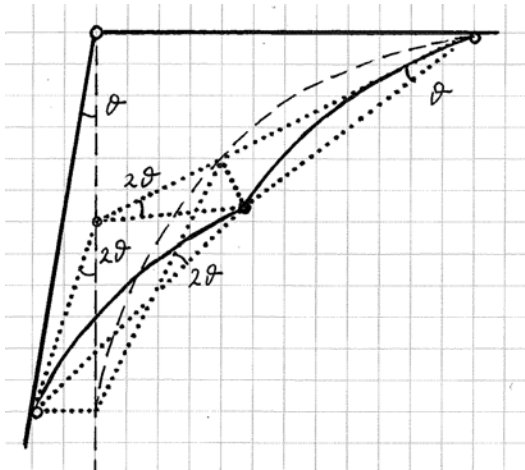
- c** In plastic analysis of plates the obtained smallest upper bound often is not equal to the exact plastic collapse load. What causes this? Choose A, B, C, or D (0.5 point).

- A The true mechanism is too complicated for analysis.
- B The torsion moments have been neglected.
- C The virtual work equation gives an approximation of equilibrium (weak formulation).
- D The round-off errors made in the hand calculations.

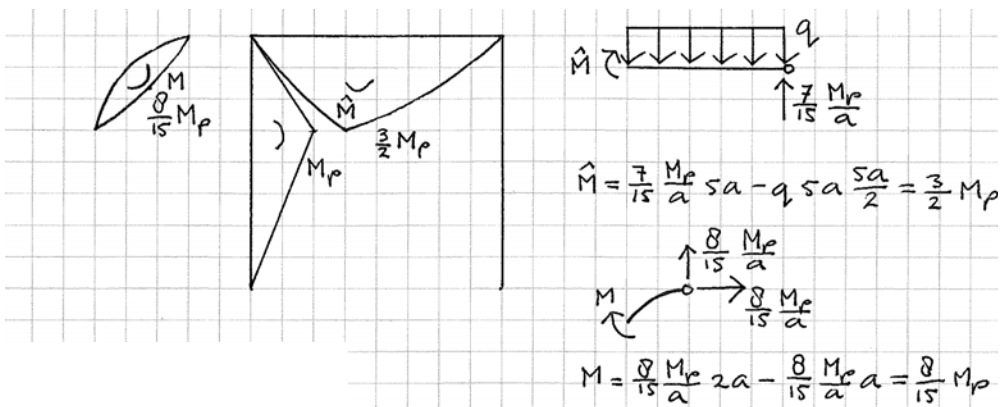
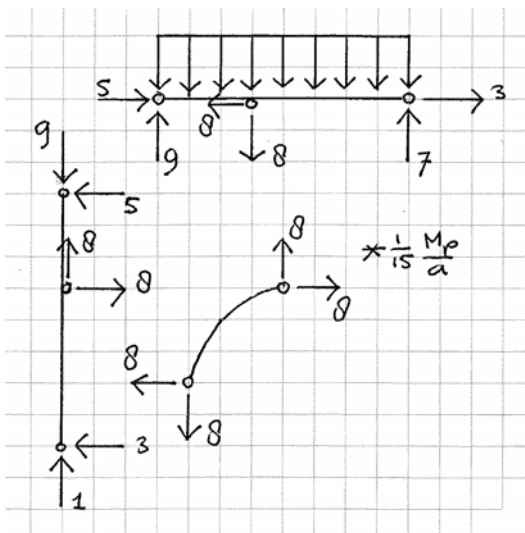
Answer to Problem 1a

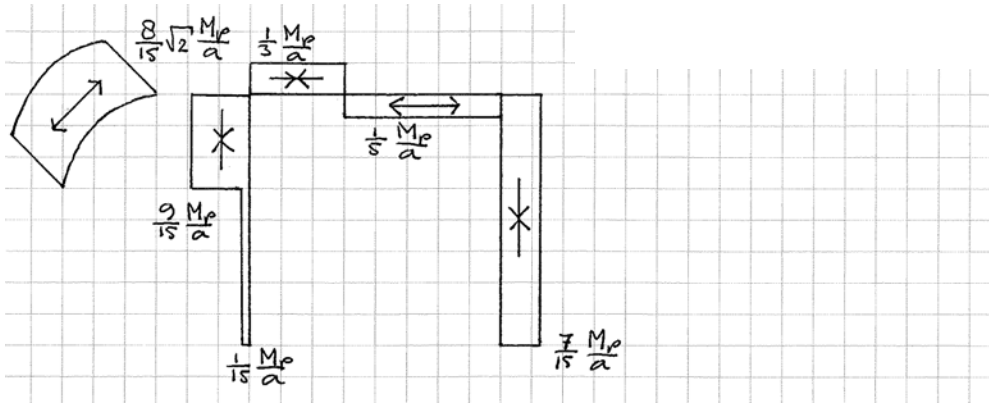


(See bracket detail at page 5.)

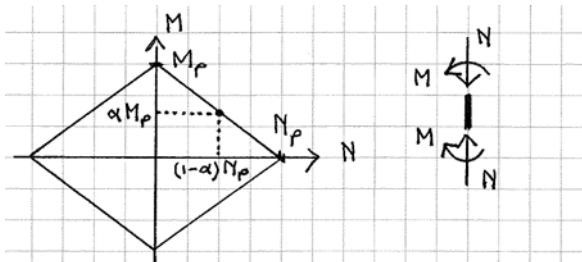


Answer to Problem 1b





Answer to problem 1c Lower-bound



$$N = \frac{1}{15} \frac{\alpha M_p}{a} = (1-\alpha) N_p = (1-\alpha) \beta \frac{M_p}{a} \Rightarrow$$

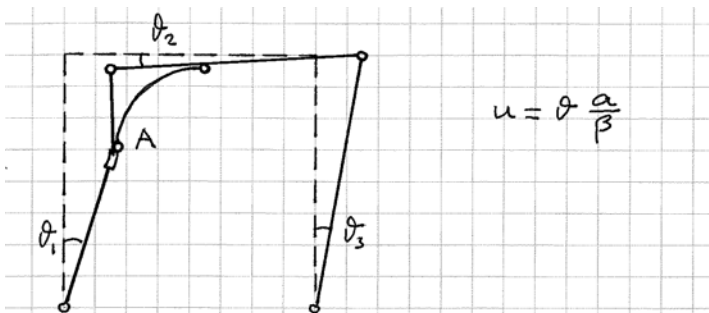
$$\frac{1}{15} \alpha = (1-\alpha) \beta \Rightarrow$$

$$\frac{1}{15} \alpha + \beta \alpha = \beta \Rightarrow$$

$$\alpha = \frac{\beta}{\frac{1}{15} + \beta} = \frac{90}{\frac{1}{15} + 90} = \frac{1350}{1351}$$

$$q = \alpha \frac{1}{15} \frac{M_p}{a^2} = \frac{90}{1351} \frac{M_p}{a^2}$$

Answer to Problem 1c Upper-bound



horizontal displacement of A $\rightarrow$

$$\theta_1 5a = \theta_3 8a + \theta_2 3a$$

vertical displacement of A $\downarrow$

$$(\theta_1 + \theta_2) \frac{a}{\beta} = \theta_2 8a$$

$$E = M_p (\theta_1 + \theta_2)$$

$$A = q \cdot 8a \cdot \theta_2 \cdot 4a + 3aq \cdot \theta_3 \cdot 8a$$

$$E = A$$

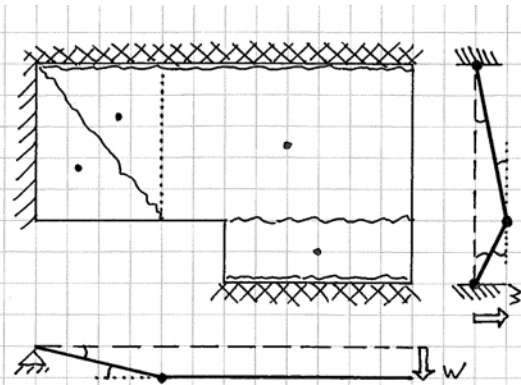
The solution to the equations is

$$\theta_1 = 719 \theta_2, \quad \theta_3 = 449 \theta_2, \quad q = \frac{90}{1351} \frac{M_p}{a^2}$$

Answer to Problem 2a

Kinematically possible are pattern C and D.

Answer to Problem 2b



$$E = m_p 6a \left(\frac{w}{2a} + \frac{w}{5a} \right) + 2m_p 5a \frac{w}{4a} + m_p 4a \frac{w}{5a} +$$

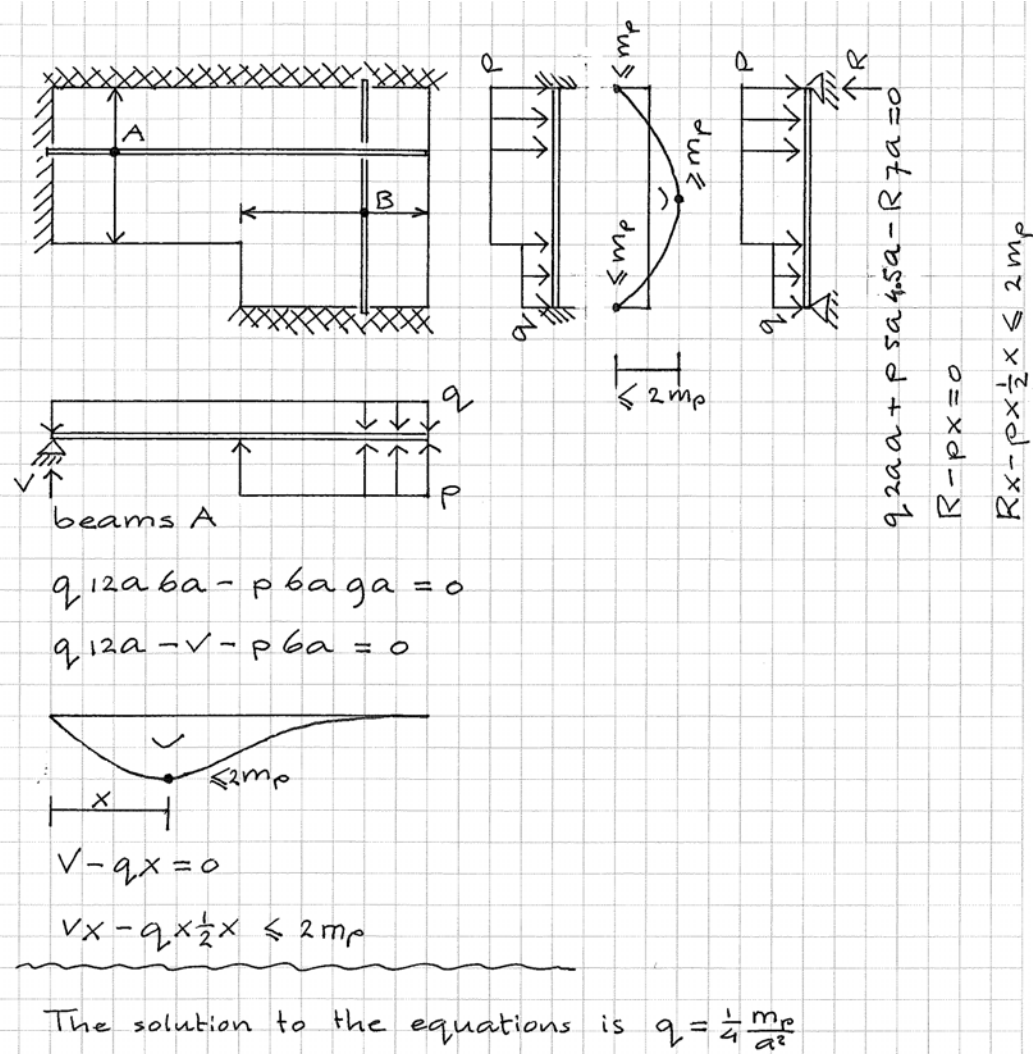
$$+ m_p 6a \frac{w}{2a} + m_p 12a \frac{w}{5a} = \frac{129}{10} m_p w$$

$$A = q \cdot 2a \cdot 6a \cdot \frac{w}{2} + q \cdot 8a \cdot 5a \cdot \frac{w}{2} + q \cdot \frac{1}{2} \cdot 4a \cdot 5a \cdot \frac{1}{3} w +$$

$$+ q \cdot \frac{1}{2} \cdot 4a \cdot 5a \cdot \frac{1}{3} w = \frac{98}{3} q a^2 w$$

$$E = A \Rightarrow q = \frac{387}{980} \frac{m_p}{a^2} \approx 0.39 \frac{m_p}{a^2}$$

Answer to Problem 2c



Answer to Problem 3a

Answer A is correct.

Answer to Problem 3b

No yielding when

$$m_{xy}^2 < \min[(m_{px} - m_{xx})(m_{py} - m_{yy}), (m'_{px} + m_{xx})(m'_{py} + m_{yy})] \quad (12.13)$$

$$5^2 < \min[(17 - 13)(0 + 8), (0 + 13)(10 - 8)]$$

$$25 < \min[32, 26]$$

Therefore, no yielding.

Answer to Problem 3c

Answer A is correct.