

Exam CT4150 Plastic Analysis of Structures
 Thursday 7 April 2011, 14:00 – 17:00 hours

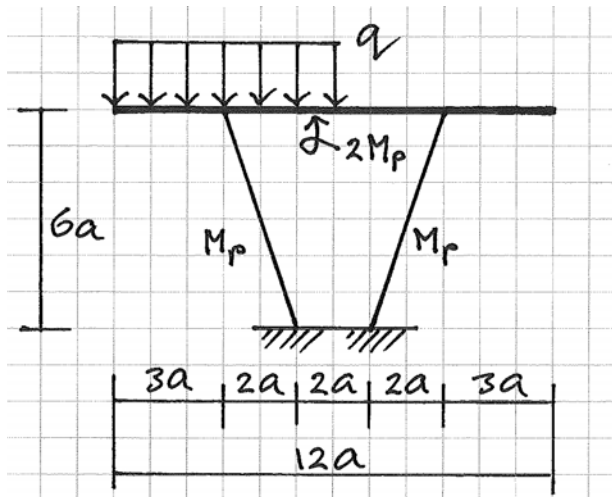


Figure 1. Frame structure

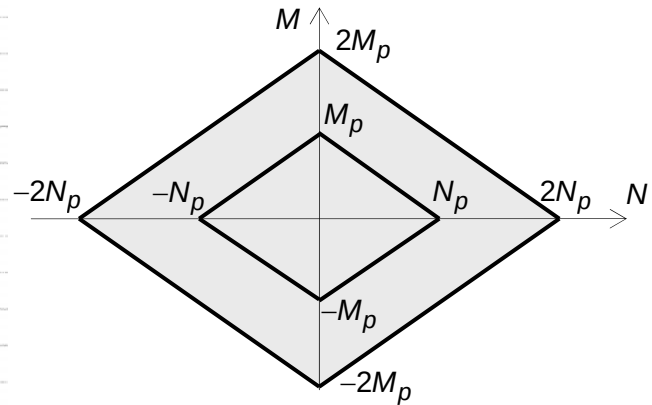


Figure 2. Yield contours

Problem 1

A frame consists of two columns and a beam (Fig. 1). The beam is two times as strong as the columns. The joints are fixed connections. The structure is loaded by a vertical load q that is evenly distributed over the left half of the beam. The relation of Figure 2 exists between the plastic moment M_p and the plastic normal force N_p .

$$N_p = \beta \frac{M_p}{a}$$

The influence of shear on the yield contour is neglected. Buckling and second order effects are not considered.

- a** Assume $\beta \rightarrow \infty$. Determine the collapse load q for all possible mechanisms. Write the collapse loads as functions of M_p and a . What is the decisive collapse load? (1.5 point)
- b** Assume $\beta \rightarrow \infty$. Draw the bending moment diagram and normal force diagram for the structure at the moment of collapse. (1.5 points)
- c** Assume $\beta = 9\sqrt{10}$. Choose one of the following problems (You need not do both).
 - Use Figure 3 to determine the largest lower-bound for q .
 - Determine the smallest upper-bound for q .
 You only need to write down the equations and not solve the equations (1.5 points).

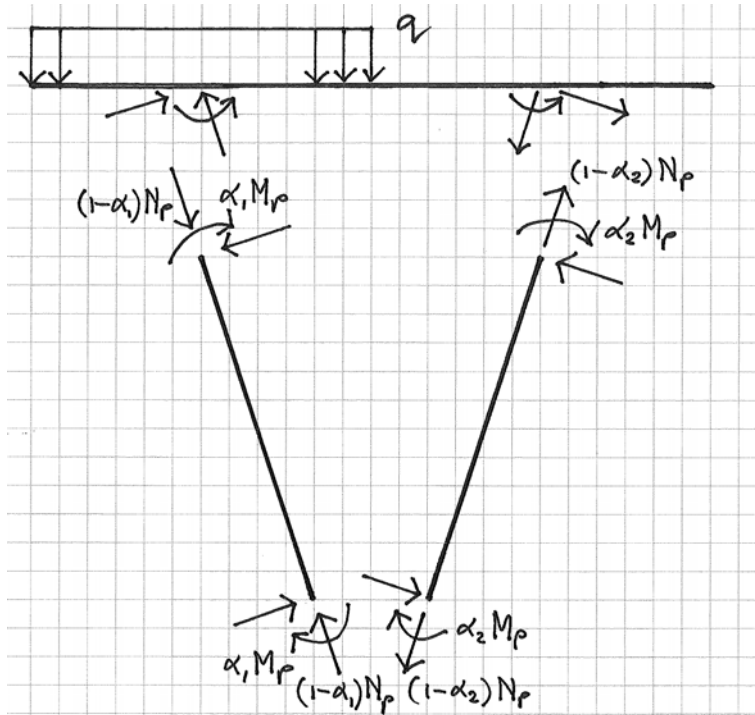


Figure 3. Equilibrium system for including M-N interaction

Problem 2

A reinforced concrete plate has two simply supported edges (Fig. 4). It carries an evenly distributed load q over half of the plate (shaded area). The plate is homogeneous and orthotropic.

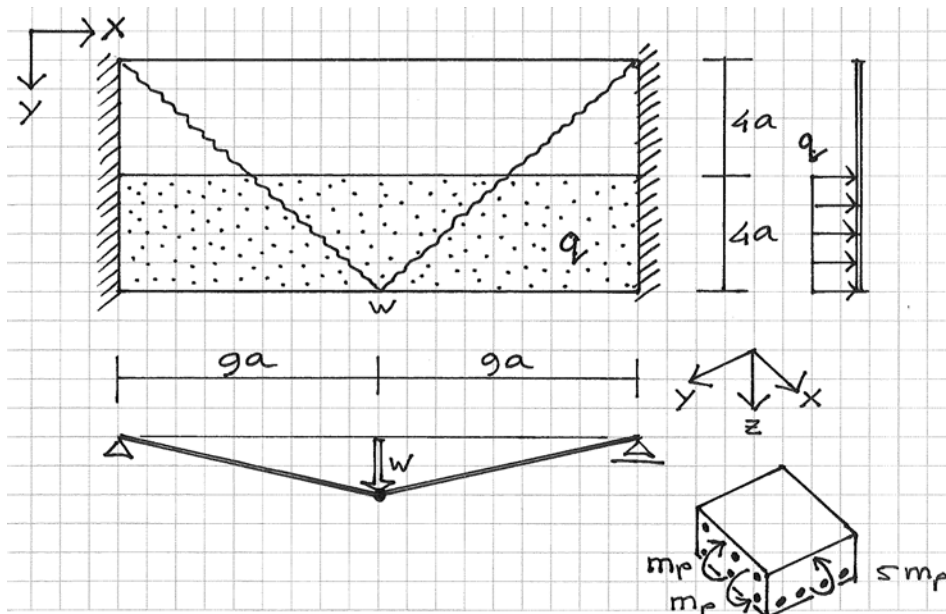


Figure 4. Plate dimensions and reinforcement

- a Consider the yield line patterns of Figure 5. Which of these patterns give kinematically possible mechanisms? (1 point)

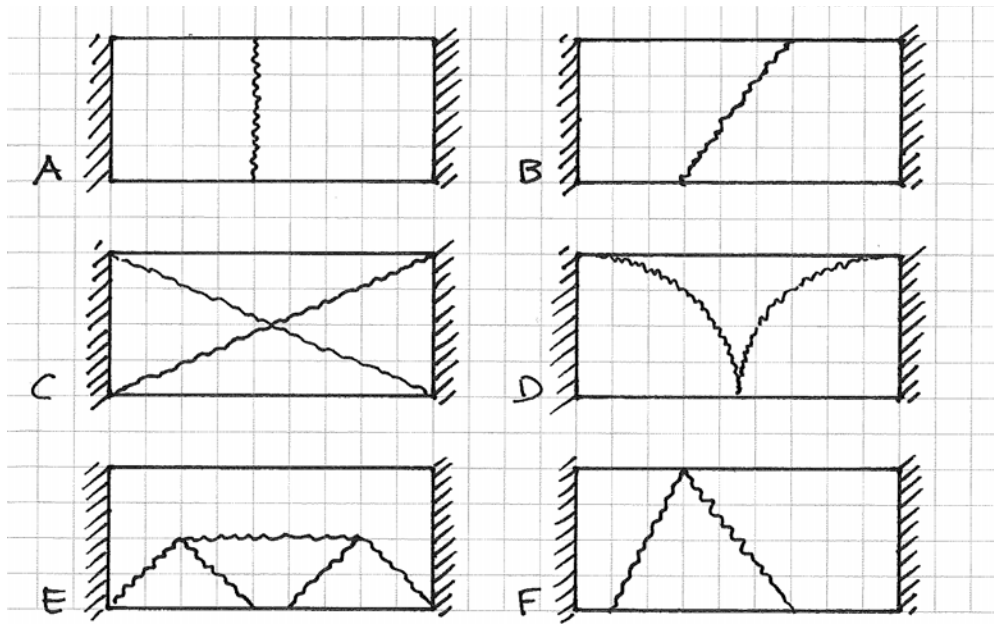


Figure 5. Yield line patterns of problem 2a

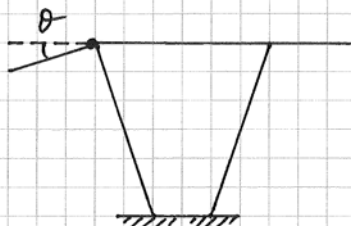
- b** Consider the yield line pattern of Figure 4. Determine an upper bound for q expressed in m_p and a (1.5 point).
- c** Determine the largest lower-bound for q using torsion free beams ($m_{xy} = 0$) (1.5 point).

Problem 3

- a** The upper bound theorem of plasticity theory ... Choose A, B, C, or D (0.5 point).
- A ... is generally believed to be true but it has not been mathematically proved as yet.
 - B ... has been mathematically proved by W. Prager in 1950.
 - C ... has been confirmed by many experiments but is unlikely to be proved mathematically because clear exceptions have been found, such as simply supported reinforced concrete beams.
 - D ... has been formulated and proved by Vitruvius (~ 85 – 20 BC).
- b** Which yield condition depends on two experimentally determined numbers? Choose A, B, C, or D (0.5 point).
- A Von Mises yield condition,
 - B Tresca yield condition,
 - C Mohr-Coulomb yield condition,
 - D Saint Venant yield condition.

- c Temperature stresses do not influence the strength of ductile structures. Why is this?
Choose A, B, C or D (0.5 point).
- A Because temperature stresses are small and can be neglected.
 - B Because the plastic hinges make the structure statically determinate and in a statically determinate structure, temperature stresses do not occur.
 - C Because temperature loading is compensated by creep. In the short term it might have some influence but in the long term it has not.
 - D Temperature stress influences the deformations and if the applied dilatations are not sufficient it can lead to premature failure.

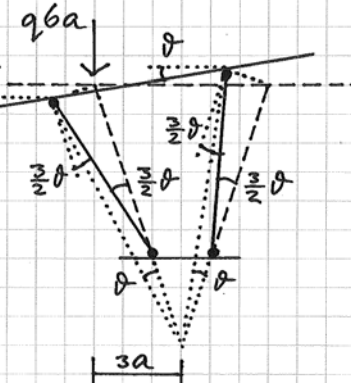
Answer to Problem 1a



$$E = 2 M_p \theta$$

$$A = q 3a \theta \frac{3}{2} a$$

$$E = A \Rightarrow q = \frac{4}{9} \frac{M_p}{a^2}$$

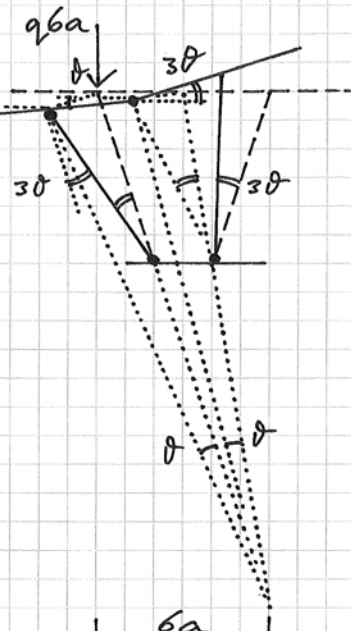


$$E = M_p \frac{3}{2} \theta + M_p (\frac{3}{2} \theta - \theta) + M_p (\frac{3}{2} \theta - \theta) + M_p \frac{3}{2} \theta$$

$$A = q 6a \theta 3a$$

$$E = A \Rightarrow \boxed{q = \frac{2}{9} \frac{M_p}{a^2}}$$

decisive

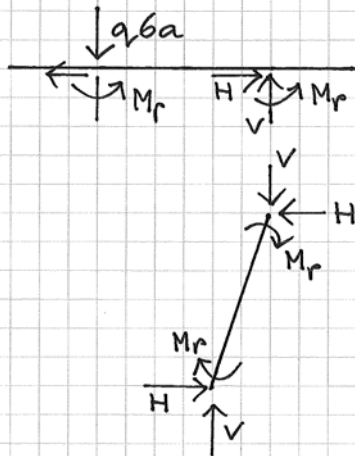


$$E = M_p 3\theta + M_p (3\theta - \theta) + 2 M_p (3\theta - \theta) + M_p 3\theta$$

$$A = q 6a \theta 6a$$

$$E = A \Rightarrow q = \frac{1}{3} \frac{M_p}{a^2}$$

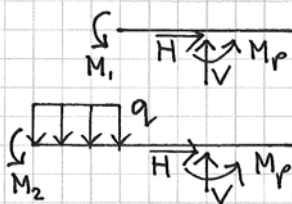
Answer to Problem 1b



$$2M_p + V6a = 0$$

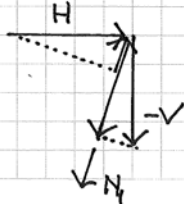
$$2M_p + V2a - H6a = 0$$

$$\left\{ \begin{array}{l} H = \frac{2}{3} \frac{M_p}{a} \\ V = -\frac{1}{3} \frac{M_p}{a} \end{array} \right.$$

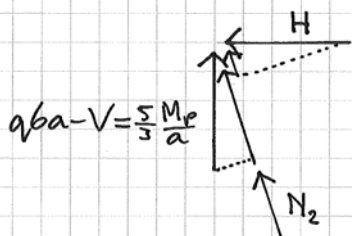


$$M_1 = -V3a - M_p = 0$$

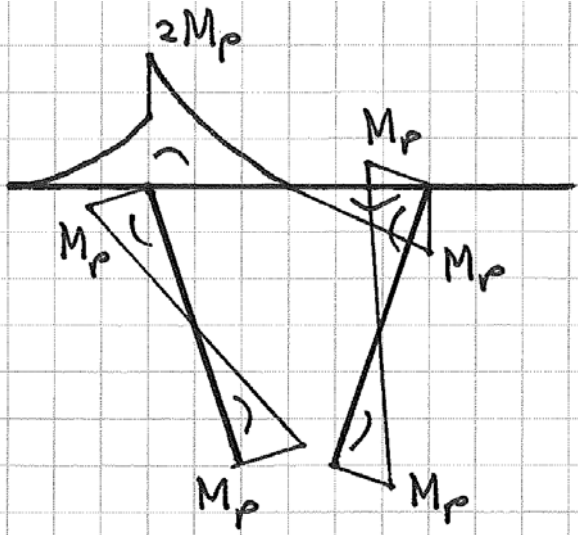
$$M_2 = q3a \frac{3a}{2} - V6a - M_p = 2M_p$$

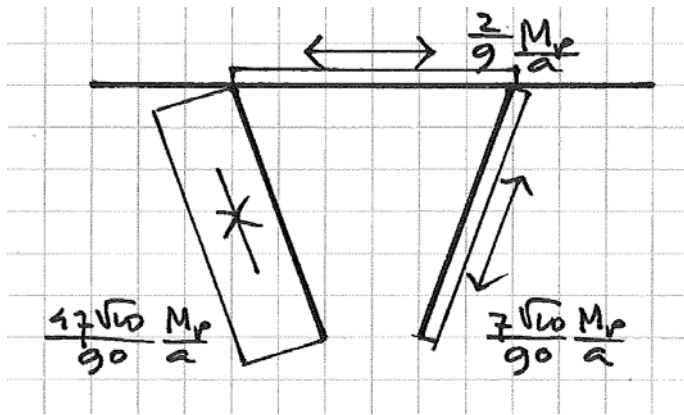


$$N_1 = -V \frac{3}{\sqrt{10}} - H \frac{1}{\sqrt{10}} = \frac{7\sqrt{10}}{90} \frac{M_p}{a}$$



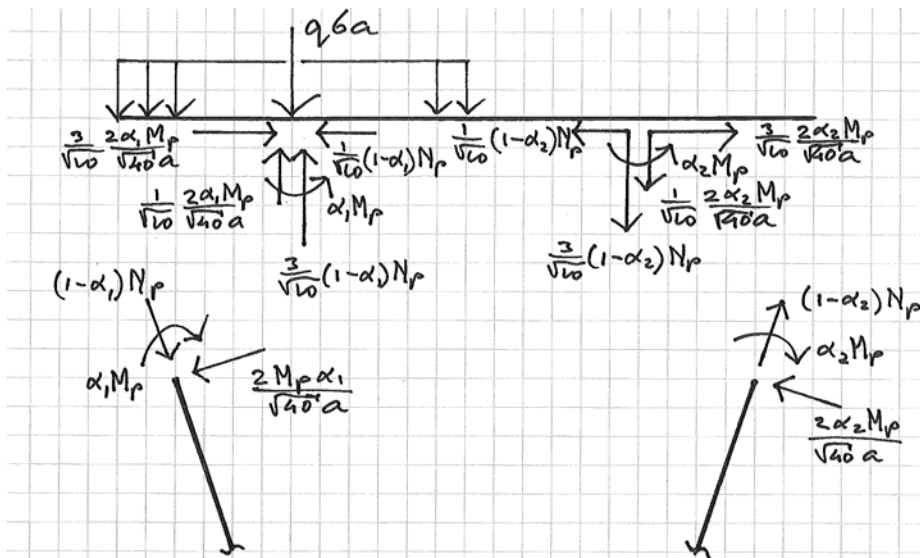
$$N_2 = H \frac{1}{\sqrt{10}} + \frac{5}{3} \frac{M_p}{a} \frac{3}{\sqrt{10}} = \frac{47\sqrt{10}}{90} \frac{M_p}{a}$$





Answer to Problem 1c

lower-bound



verticaal evenwicht ↓

$$q \cdot 6a - \frac{1}{\sqrt{10}} \frac{2\alpha_1 M_p}{\sqrt{40}a} - \frac{3}{\sqrt{10}} (1-\alpha_1) N_p + \frac{3}{\sqrt{10}} (1-\alpha_2) N_p + \frac{1}{\sqrt{10}} \frac{2\alpha_2 M_p}{\sqrt{40}a} = 0$$

horizontaal evenwicht →

$$\frac{3}{\sqrt{10}} \frac{2\alpha_1 M_p}{\sqrt{40}a} - \frac{1}{\sqrt{10}} (1-\alpha_1) N_p - \frac{1}{\sqrt{10}} (1-\alpha_2) N_p + \frac{3}{\sqrt{10}} \frac{2\alpha_2 M_p}{\sqrt{40}a} = 0$$

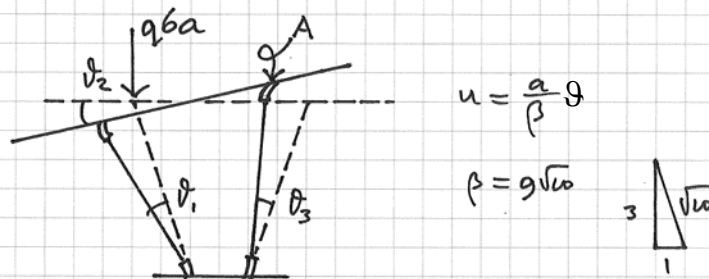
momenten evenwicht ↺

$$\alpha_1 M_p + \alpha_2 M_p - 6a \left[\frac{3}{\sqrt{10}} (1-\alpha_2) N_p + \frac{1}{\sqrt{10}} \frac{2\alpha_2 M_p}{\sqrt{40}a} \right] = 0$$

The solution to the equations is

$$\alpha_1 = \frac{7870}{8339}, \quad \alpha_2 = \frac{8270}{8339}, \quad q = \frac{20}{93} \frac{M_p}{a^2}$$

upper-bound



horizontal displacement of A $\leftarrow$

$$-\theta_1 \frac{a}{\beta} \frac{1}{\sqrt{10}} + \theta_1 6a - (\theta_1 - \theta_2) \frac{a}{\beta} \frac{1}{\sqrt{10}} = -\theta_3 \frac{a}{\beta} \frac{1}{\sqrt{10}} + \theta_3 6a - (\theta_3 - \theta_2) \frac{a}{\beta} \frac{1}{\sqrt{10}}$$

vertical displacement of A $\uparrow$

$$-\theta_1 \frac{a}{\beta} \frac{3}{\sqrt{10}} - \theta_1 2a - (\theta_1 - \theta_2) \frac{a}{\beta} \frac{3}{\sqrt{10}} + \theta_2 6a = \theta_3 \frac{a}{\beta} \frac{3}{\sqrt{10}} + \theta_3 2a + (\theta_3 - \theta_2) \frac{a}{\beta} \frac{3}{\sqrt{10}}$$

$$E = M_p \theta_1 + M_p (\theta_1 - \theta_2) + M_p (\theta_3 - \theta_2) + M_p \theta_3$$

$$A = q 6a \left[\theta_1 \frac{a}{\beta} \frac{3}{\sqrt{10}} + \theta_1 2a + (\theta_1 - \theta_2) \frac{a}{\beta} \frac{3}{\sqrt{10}} \right]$$

$$E = A$$

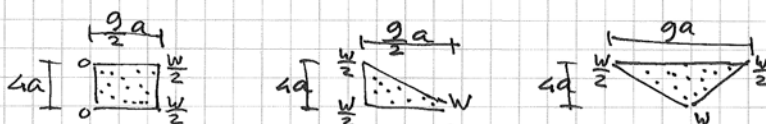
The solution to the equations is

$$\theta_1 = \frac{91}{62} \theta_2, \quad \theta_3 = \frac{91}{62} \theta_2, \quad q = \frac{20}{93} \frac{M_p}{a^2} \approx 0.215 \frac{M_p}{a^2}$$

Answer to Problem 2a

A, C, E, F

Answer to Problem 2b



$$A = 2 \left\{ 4a \frac{g}{2} a q \frac{1}{4} w + \frac{1}{2} 4a \frac{g}{2} a q \frac{2}{3} w \right\} + \frac{1}{2} 4a g a q \frac{2}{3} w$$

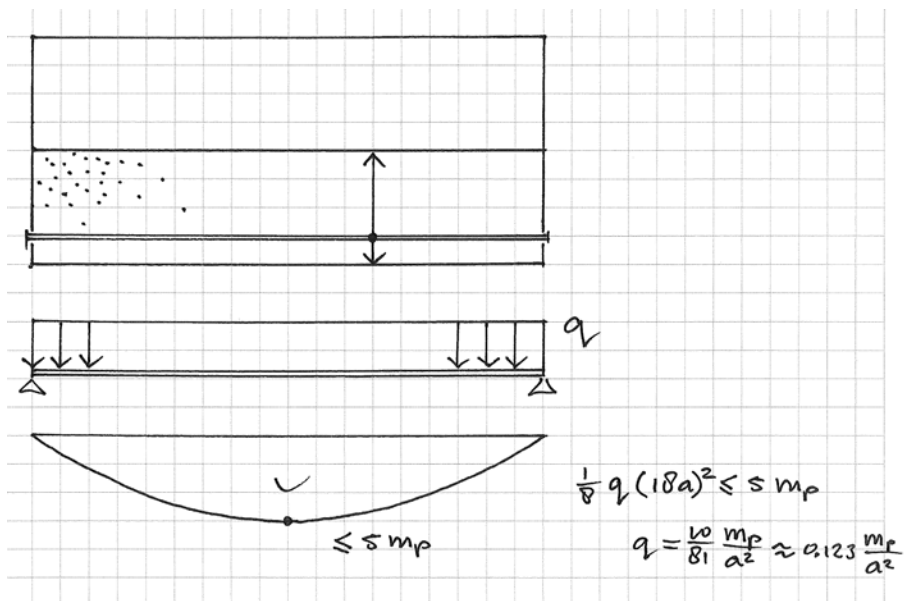
$$= 33 a^2 q w$$

$$E = 2 \left\{ 5 m_p \frac{g a}{8 a} \frac{w}{g a} + m_p g a \frac{w}{8 a} \right\}$$

$$= \frac{401}{36} m_p w$$

$$A = E \Rightarrow q = \frac{401}{1108} \frac{m_p}{a^2} \approx 0.338 \frac{m_p}{a^2}$$

Answer to Problem 2c



Answer to Problem 3a

B

Answer to Problem 3b

C

Answer to Problem 3c

B